

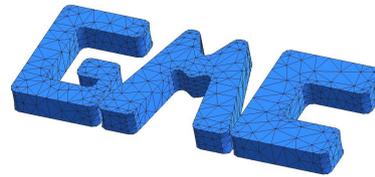
Railway Bridges for High Speed Lines: Dynamic Behaviour and Risk

Analysis and Risk Management in Production Activities
CERUP, Porto 2007

José M.^a Goicolea

Computational Mechanics Group (<http://w3.mecanica.upm.es>)
Escuela de Ingenieros de Caminos,
Technical University of Madrid

Porto, 10 oct 2007



Motivation
oooooooooooo

Design Considerations
oooooooooooooooooooo

Service Limit States
ooooooo

Concluding Remarks
oo

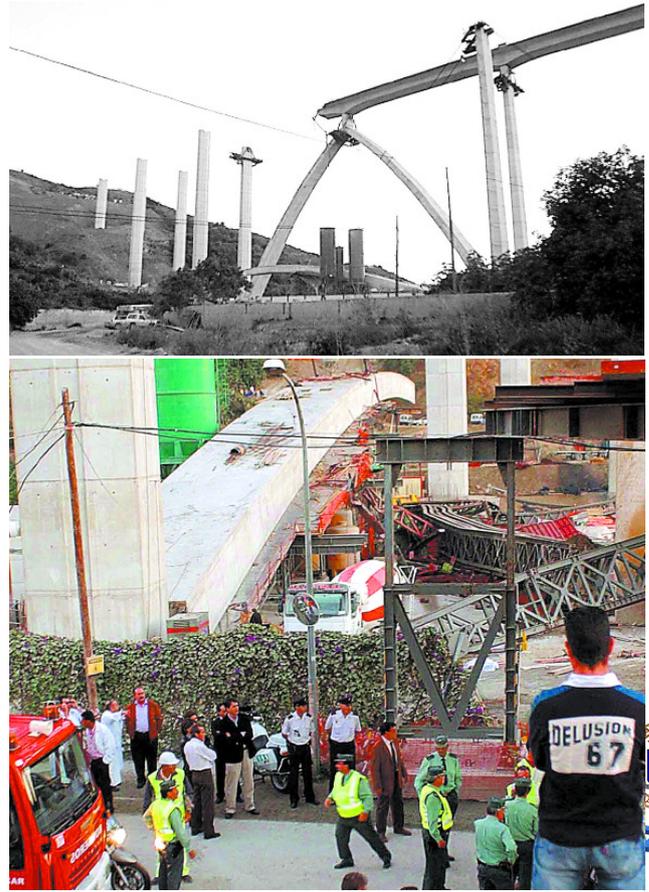
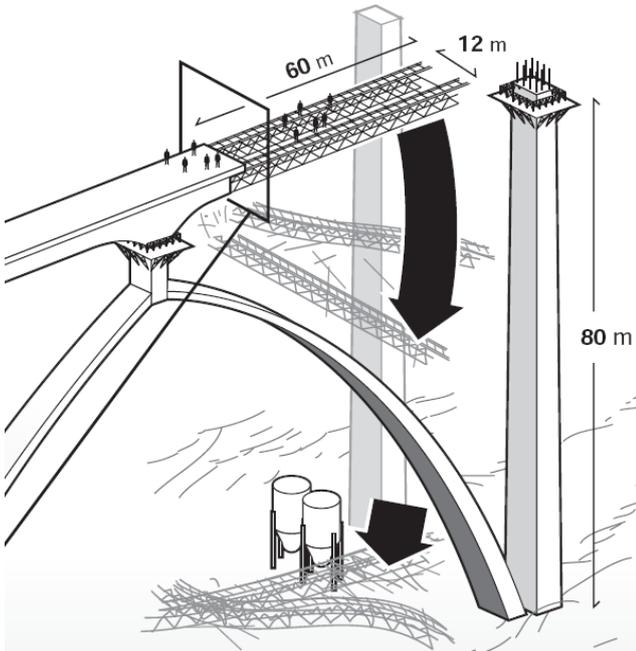
Contents

- 1 Motivation
 - Recent Events
 - Some Dynamic Phenomena
 - Dynamic Effects from HS Rail Traffic Actions
- 2 Design Considerations to Evaluate and Reduce Risk
 - Types of HS Trains
 - Issues related to bridges
- 3 Service Limit States: Risk to Traffic
 - Design requirements for traffic safety
 - Track-Bridge Interaction
- 4 Concluding Remarks



Almuñécar, Granada (Spain)

Road bridge under construction
Collapse of scaffold (7 nov 2005):
6 workers dead



Railway Bridges for High Speed Lines: Dynamic Behaviour and Risk

José M.^a Goicolea

O Carballiño, Orense (Spain)



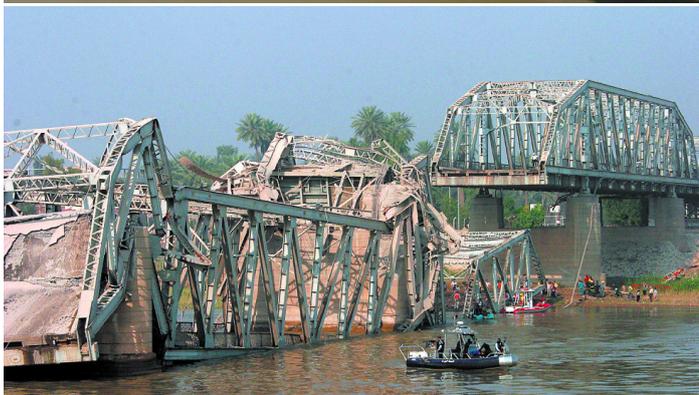
HSR bridge under construction
Incrementally launched deck
(pushing)
7 sep 2007
1 worker dead

Railway Bridges for High Speed Lines: Dynamic Behaviour and Risk

José M.^a Goicolea



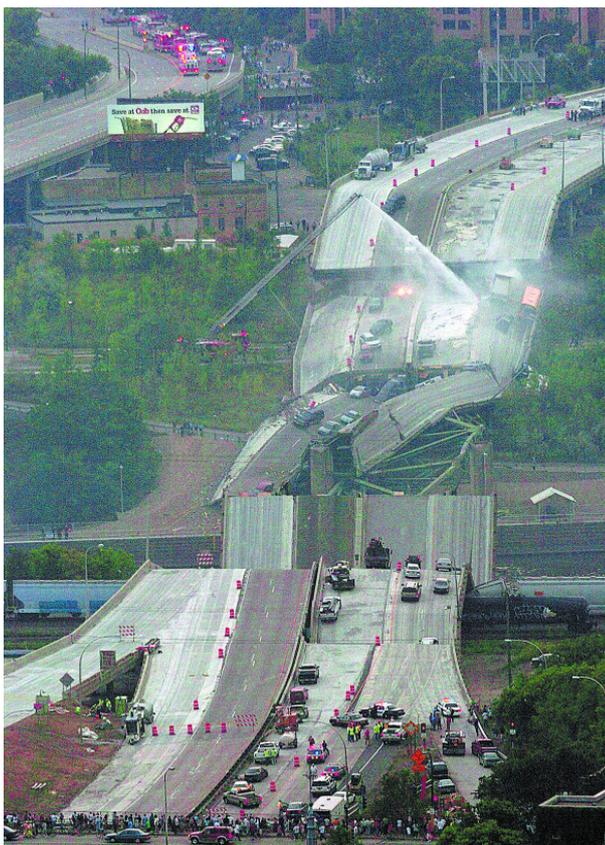
Baghdad, Sarafiya Bridge



Main urban road bridge
Truck bomb (12 apr 2007)
≈ 10 people dead



Minneapolis



Main urban road bridge
2 aug 2007: Brittle collapse;
13 people dead



Aeroelastic induced vibrations



Tacoma Narrows 1940



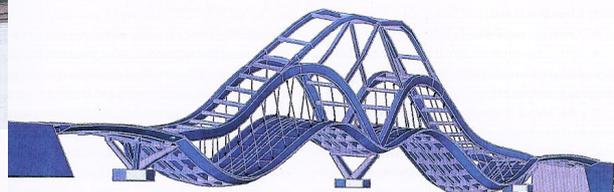
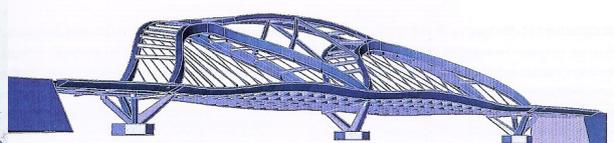
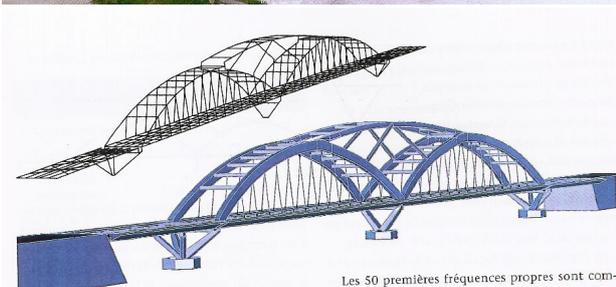
Alconétar Arch 2006

Wind Actions (IAPF 2007)

- *Normal bridges*: static wind forces
- *Singular bridges* ($L > 200$ m, ...): special dynamic study



High Speed Railway Bridges

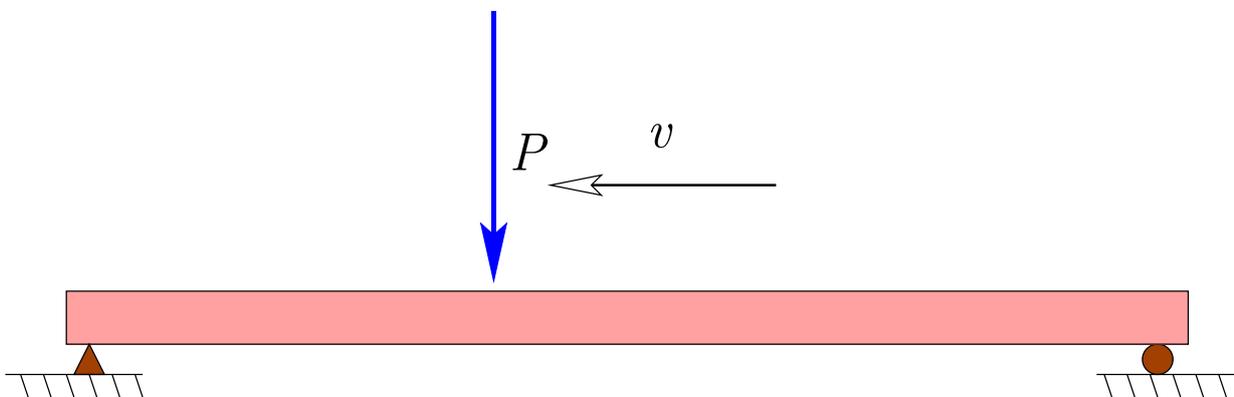


Les 50 premières fréquences propres sont com-

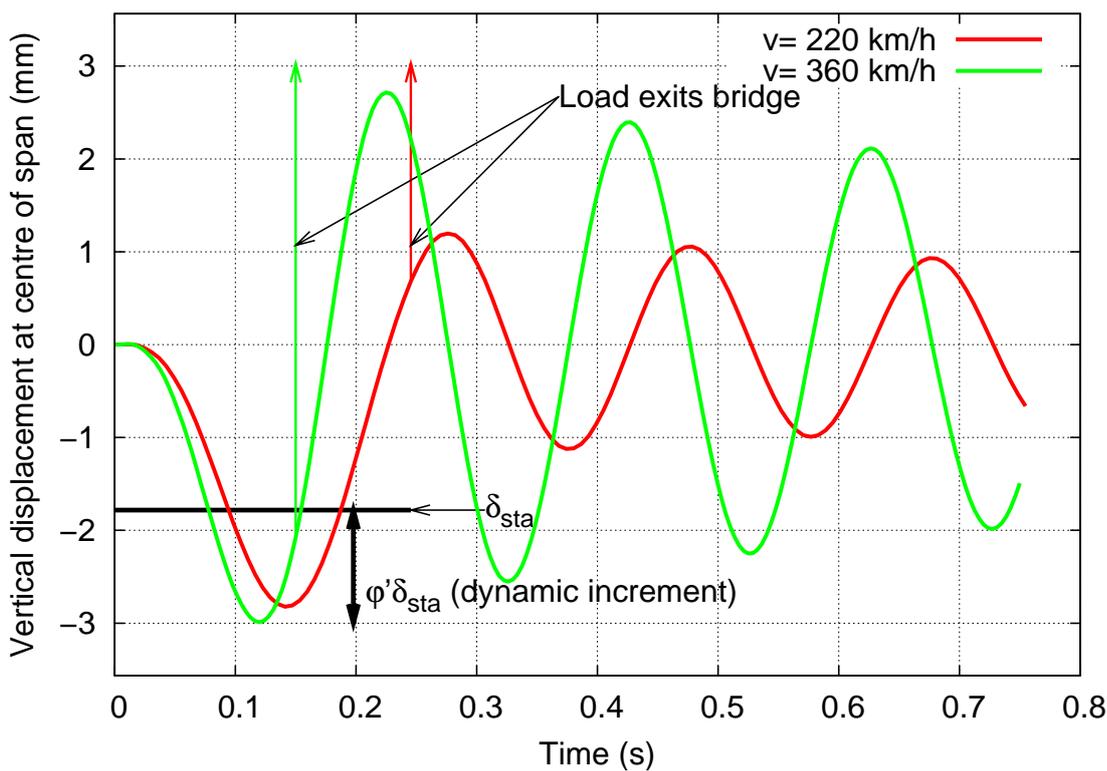
TGV Méditerranée, Donzère viaduct (arch. Marc Mimram)
 Dynamic analysis: Ing. V. de Ville de Goyet



Moving load on bridge



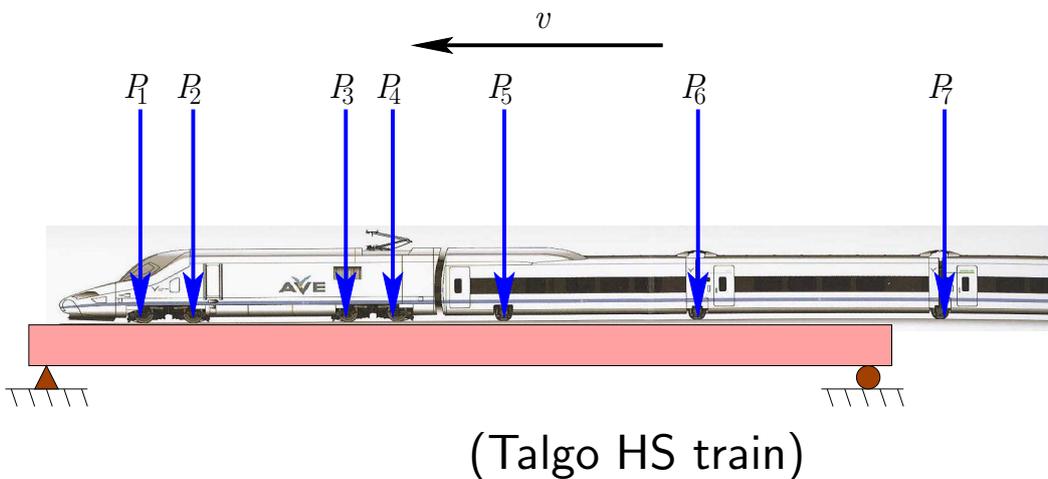
Dynamic effect of single moving load: $(1 + \varphi')\delta_{sta}$



$$L = 15 \text{ m}, \bar{m} = 15 \text{ t/m}, f_0 = 5 \text{ Hz}, P = 195 \text{ kN}, \zeta = 2\%$$

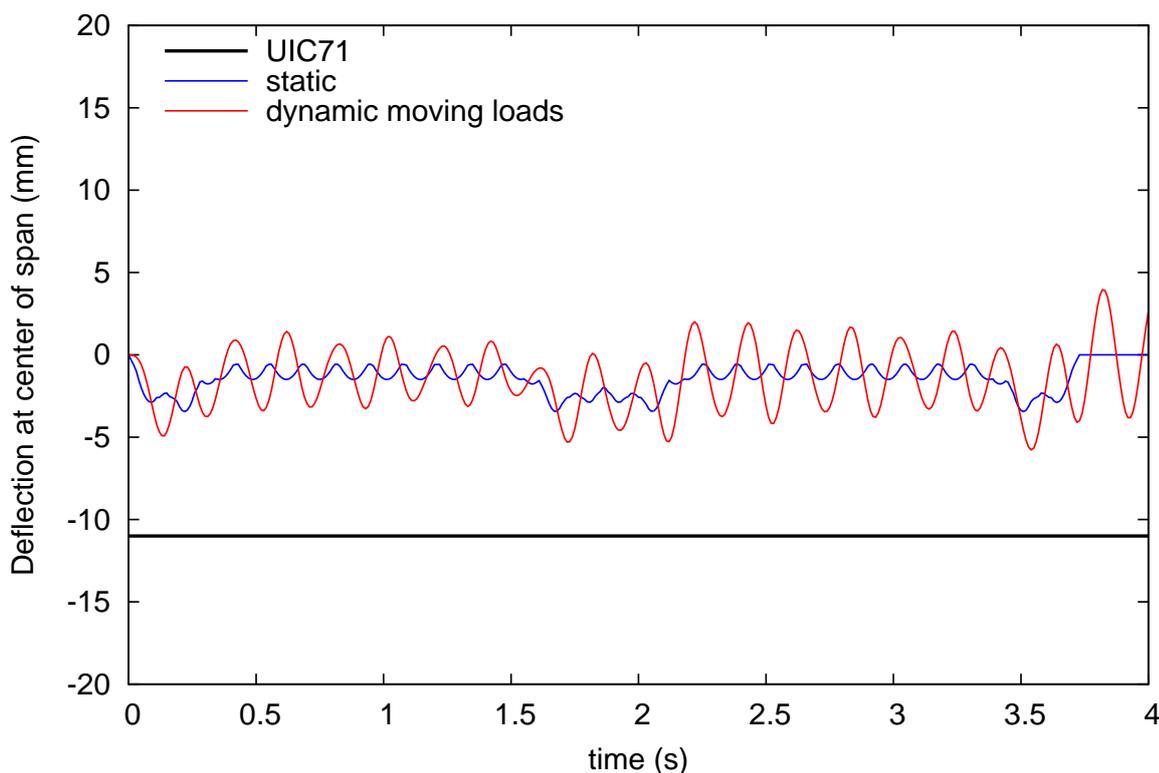


Dynamic effect of train



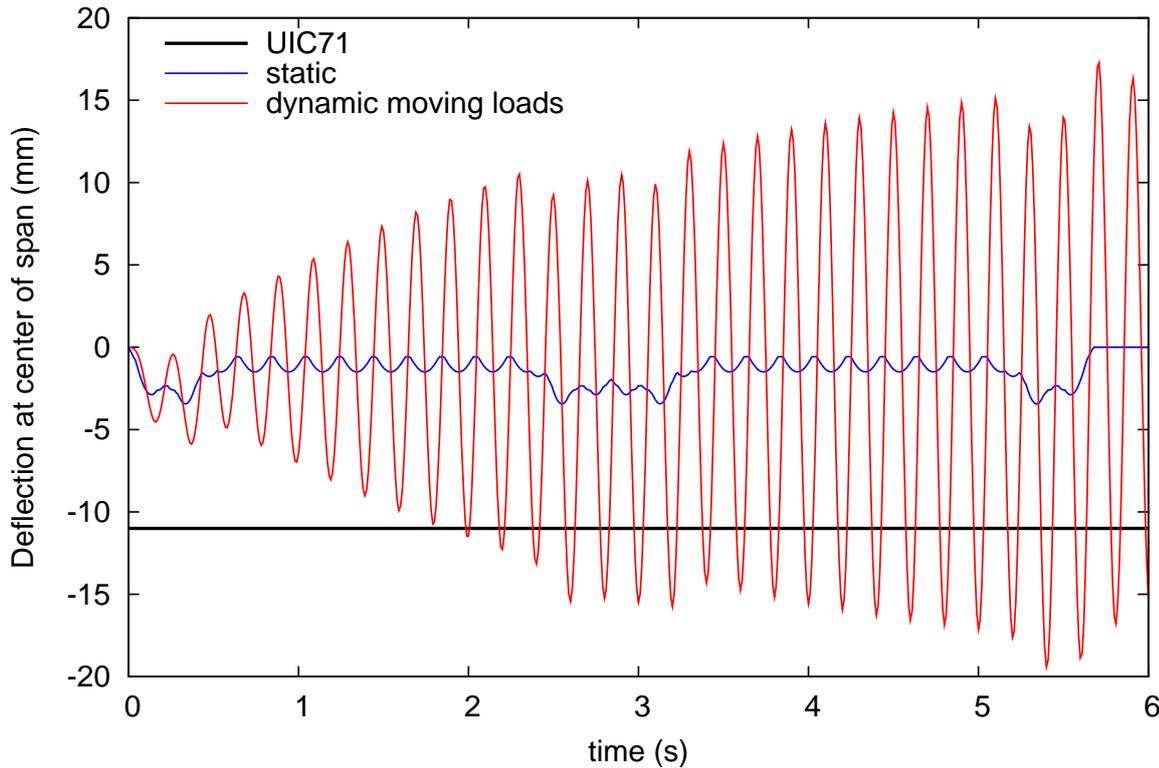
Time-history of displacements: $v = 360 \text{ km/h}$

TALGO AV $v=360 \text{ km/h}$, ERRI Bridge $L=15\text{m}$, $\zeta=0,01$; $f_0=5 \text{ Hz}$, $\lambda=13.14 \text{ m} = D$



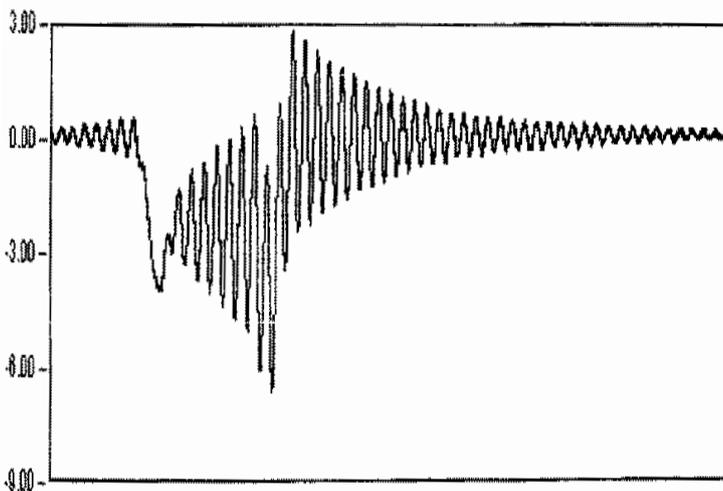
Dynamic effect of train: $v = 236.5$ km/h **resonance!**

TALGO AV $v=236.5$ km/h, ERRI Bridge $L=15$ m, $\zeta=0,01$; $f_0=5$ Hz, $\lambda=13.14$ m = D

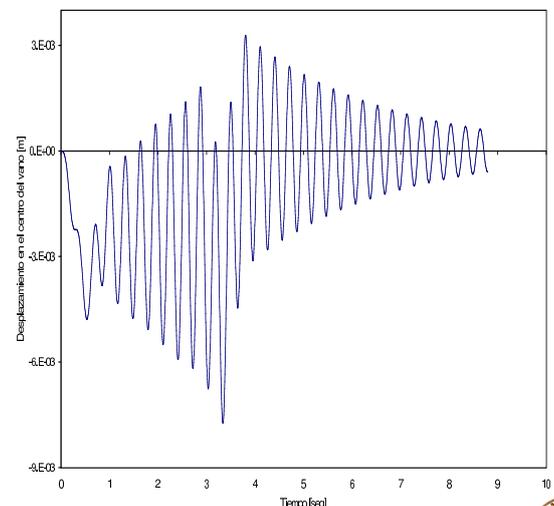


Experimental measurements: Tajo viaduct

AVE 100, simple convoy, $v = 219$ km/h



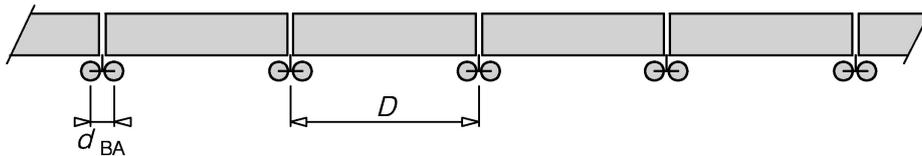
Measured displacements
[mfom 96]



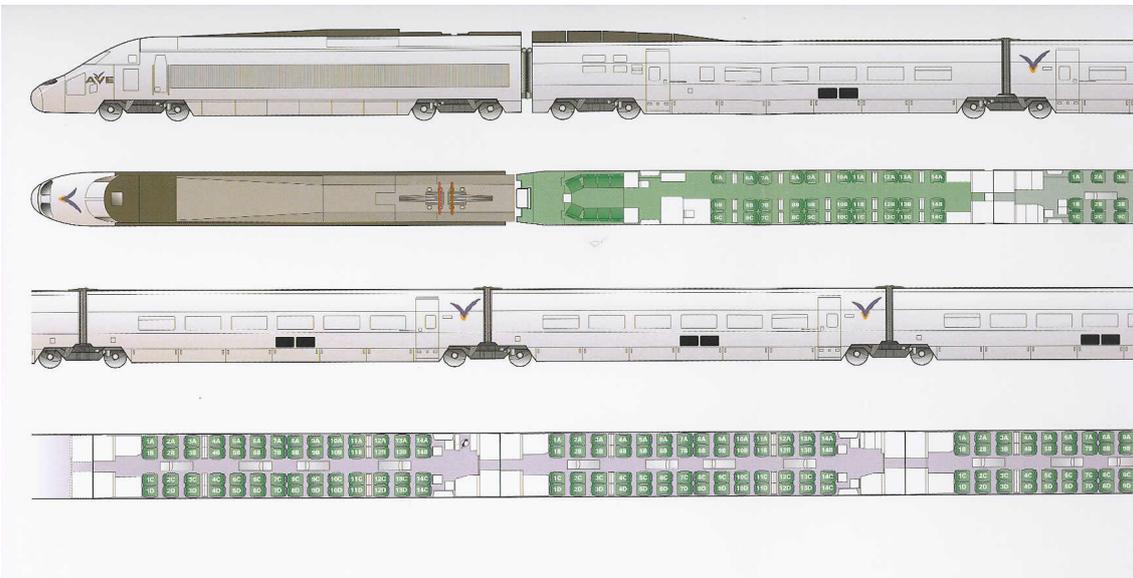
Computed displacements
[Domínguez 99]



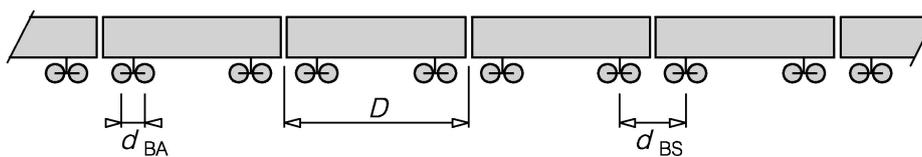
HS Trains: Articulated



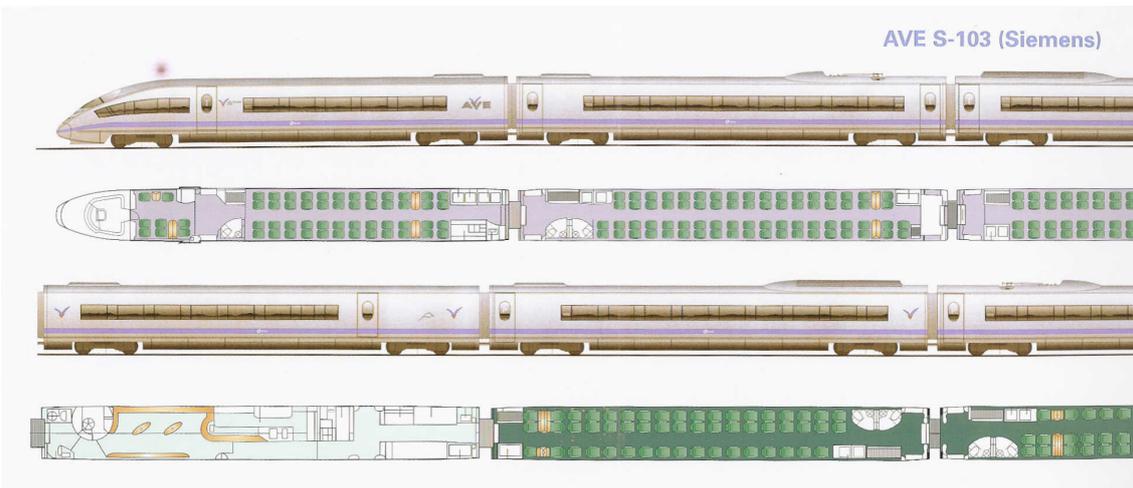
Thalys, AVE-100 and Eurostar.



HS Trains: Conventional



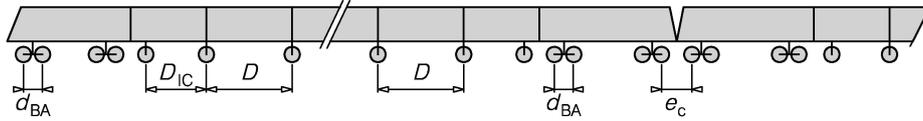
ICE2, AVE-103, Etr-y, Virgin.



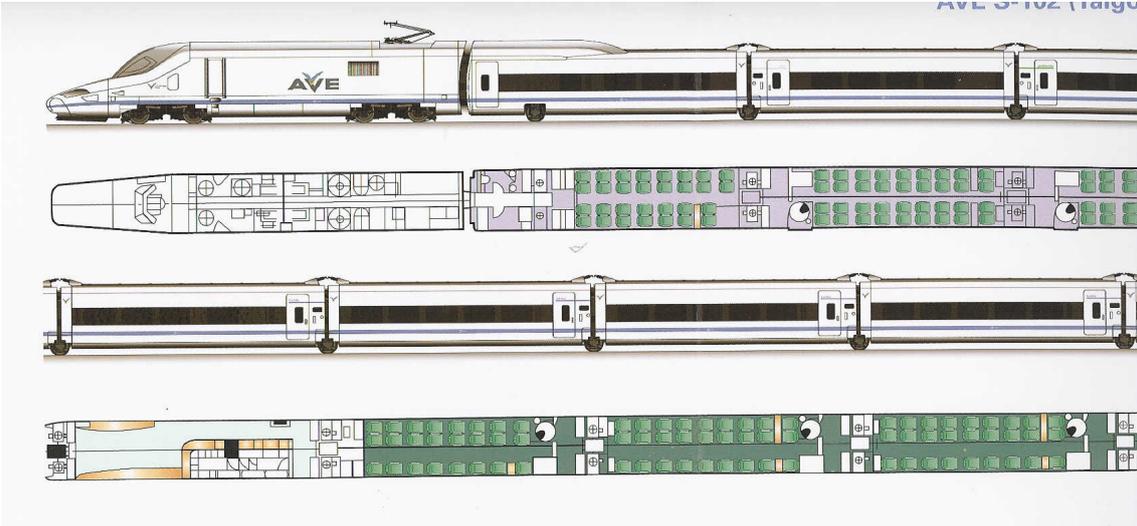
AVE S-103 (Siemens)



HS Trains: Regular

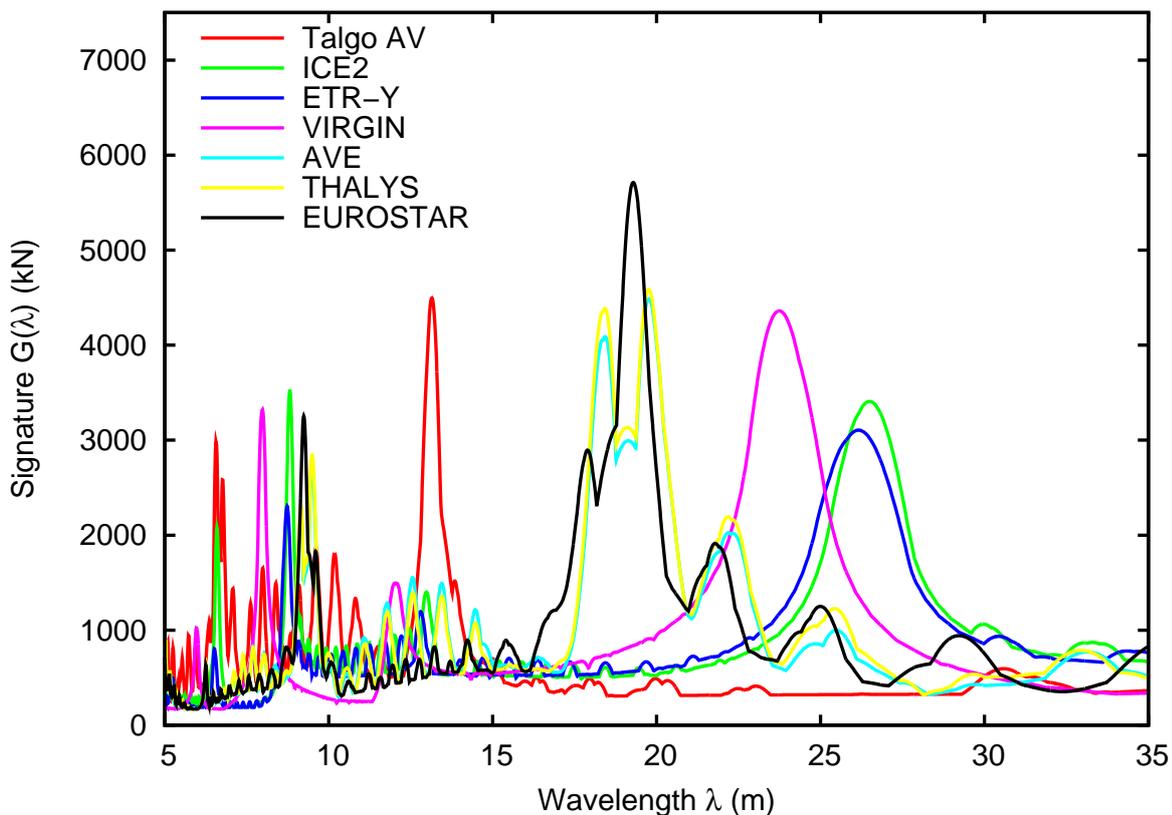


AVE-102 (TALGO).



Dynamic Signature of High Speed Trains

$G(\lambda)$ closed form expression; $\lambda = v/f_0$ wavelength



Requirements for Design

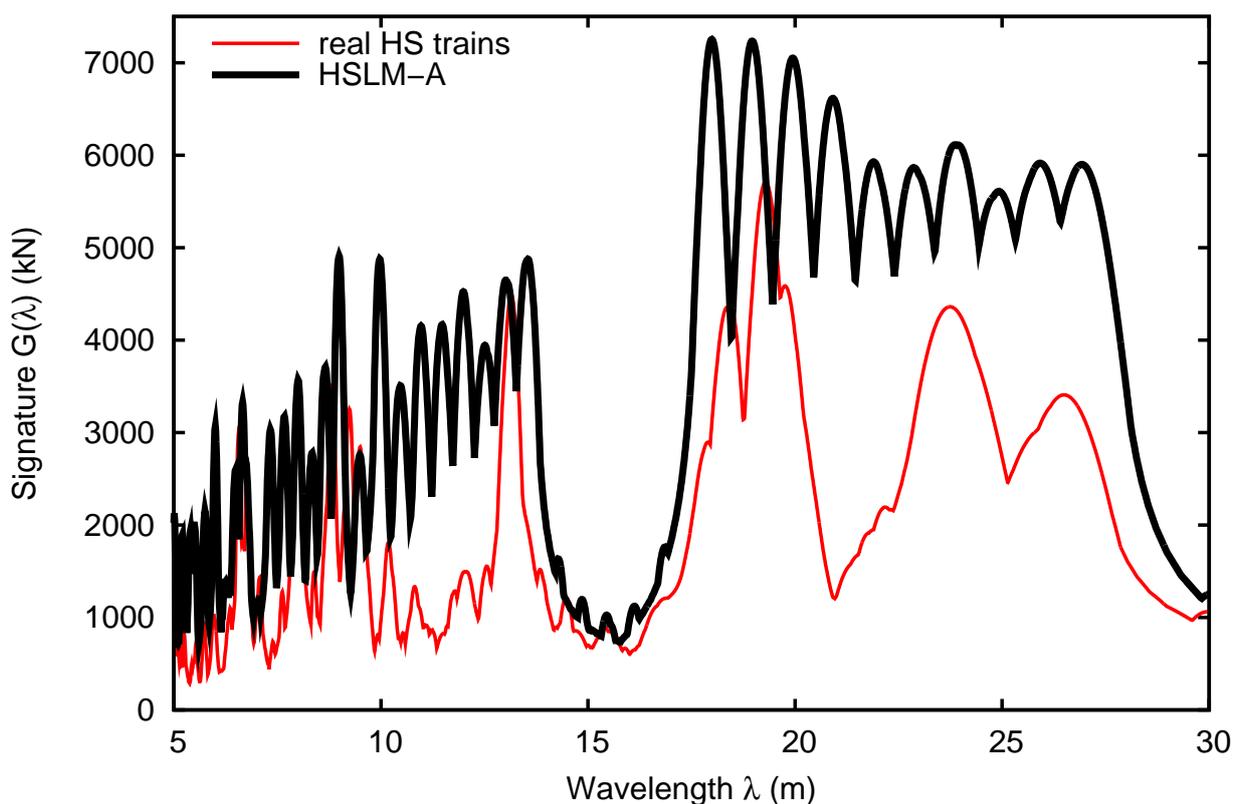
- Need to consider:**
- Dynamic effects
 - All possible circulation speeds, with 20% margin
 - All existing and foreseeable HS trains (interoperability)

- High Speed Load Model (HSLM)**
- Family of 10 normalised, *fictitious* trains
 - *Interoperability* of HS lines in Europe (TSI)

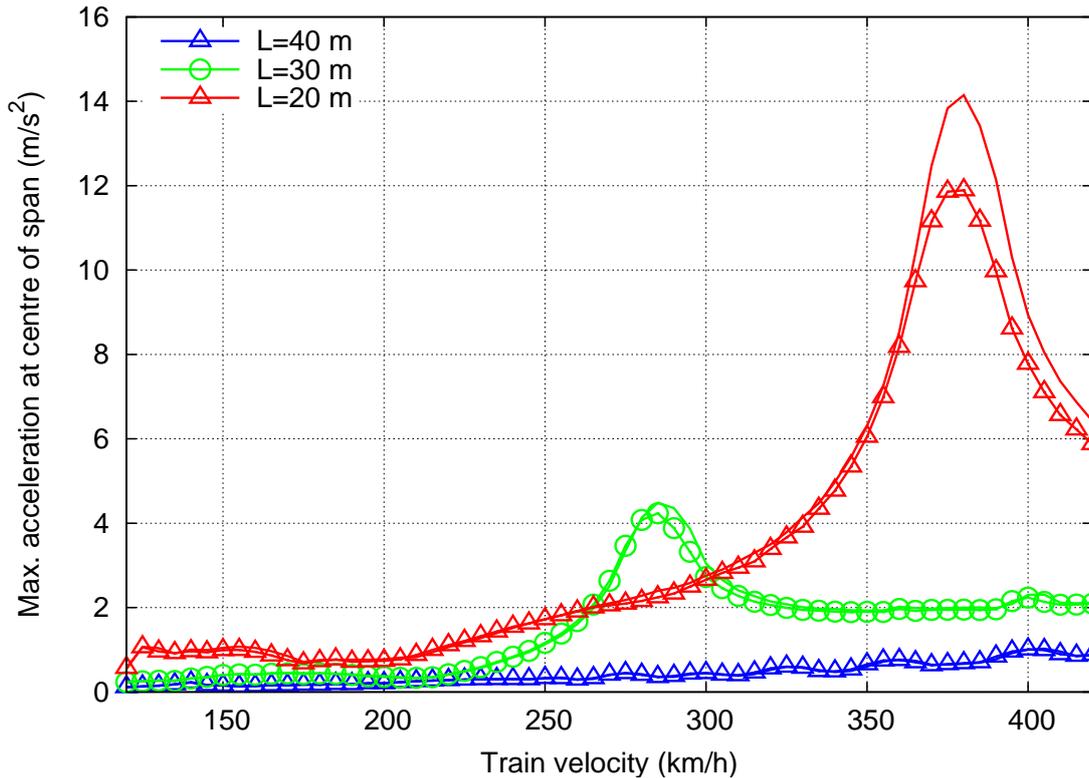


Envelope Obtained with HSLM-A Trains

Dynamic signature envelopes

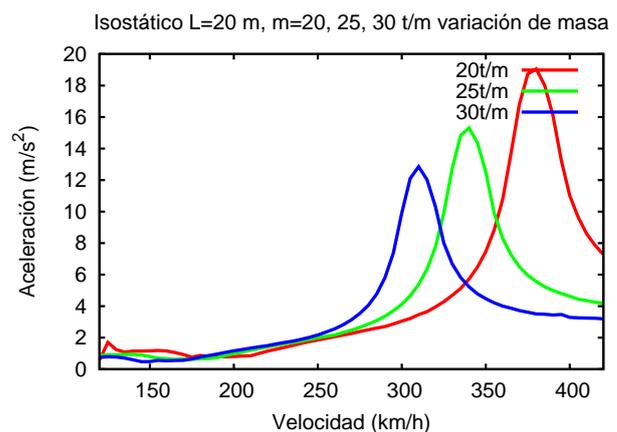
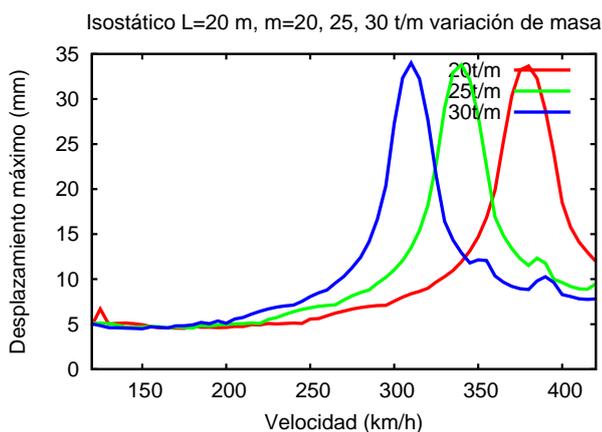


Dynamic Response for Bridges of Different Span Length



ICE2 train, acceleration at mid span

Increase Mass of Bridge

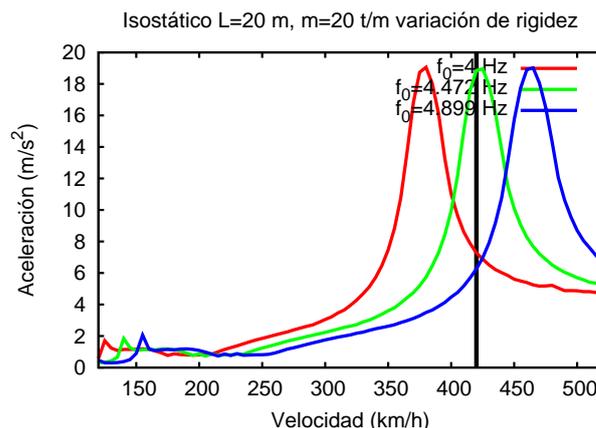
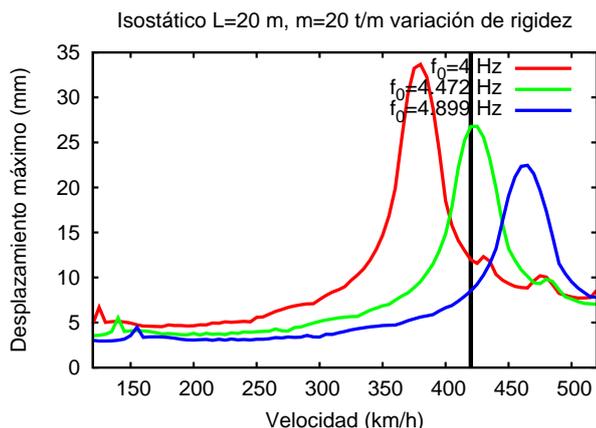


Effect on resonant response

- Frequency f_0 and critical speed v_{crit} decrease with \sqrt{m}
- Maximum displacements at resonance unchanged
- Maximum accelerations at resonance decrease



Increase Stiffness of Bridge

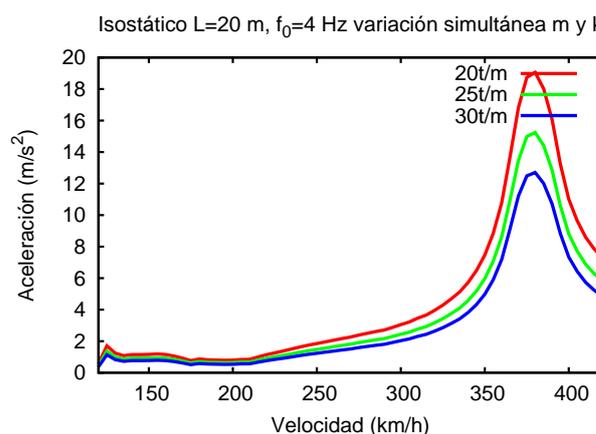
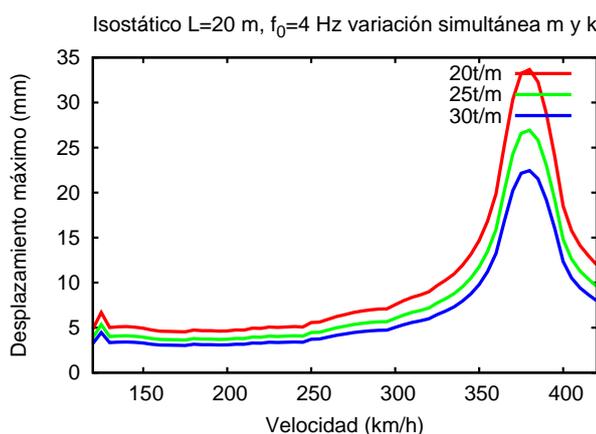


Effect on resonant response

- Frequency f_0 and critical speed v_{crit} increase with \sqrt{k} :
"expels" resonant peaks from velocity range
- Maximum displacements at resonance decrease
- Maximum accelerations at resonance unchanged



Simultaneous Increase of Mass and Stiffness



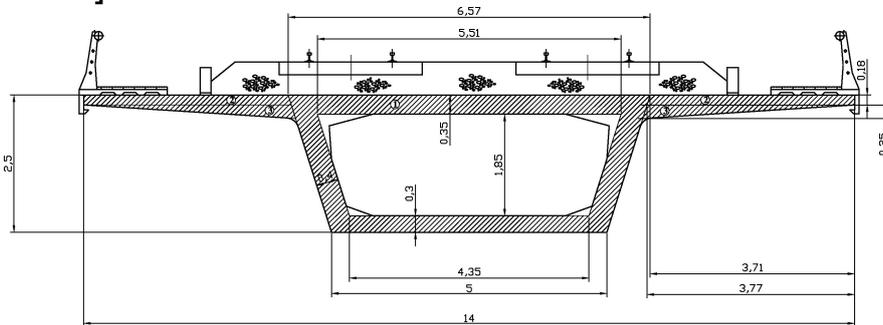
Effect on resonant response

- Frequency f_0 and critical speed v_{crit} unchanged
- Maximum displacements at resonance decrease
- Maximum accelerations at resonance decrease



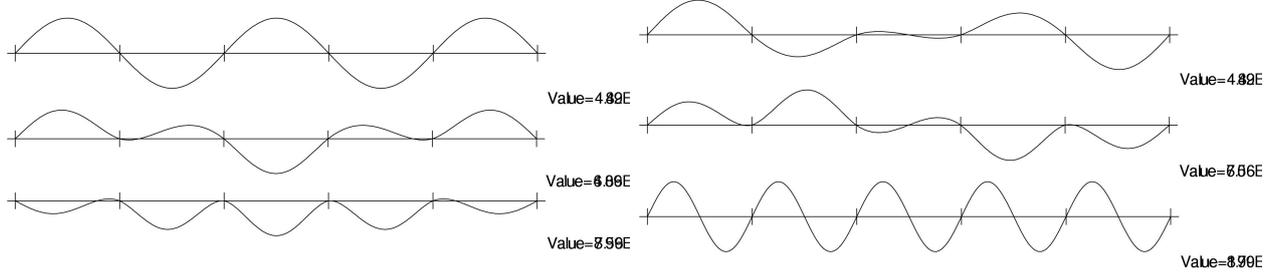
Redundant, Continuous Deck Bridges

Viaduct of *Arroyo del Salado*, continuous bridge, 30 spans of 30 m, prestressed in-situ concrete box girder [student project, B. Sanz, 2005].



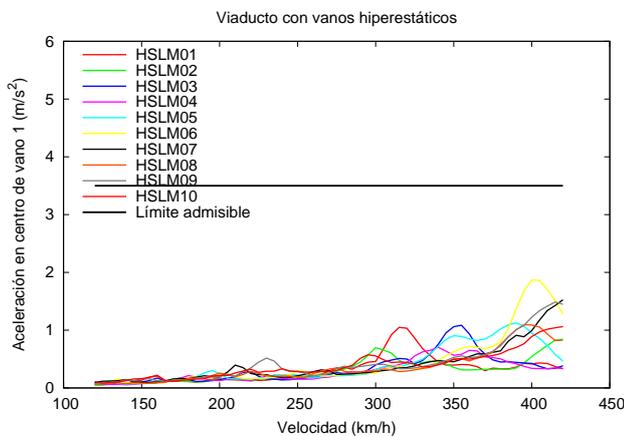
$$\frac{\text{depth}}{\text{span}} = \frac{1}{12}$$

First six modes of vibration:



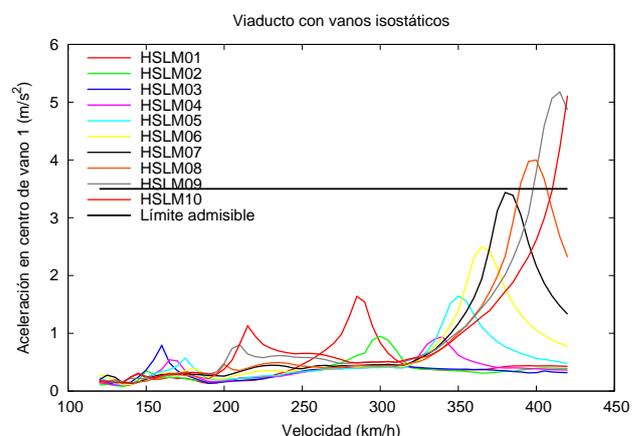
Accelerations for Continuous and Simply Supported Deck

Continuous deck



Satisfies dynamic requirements

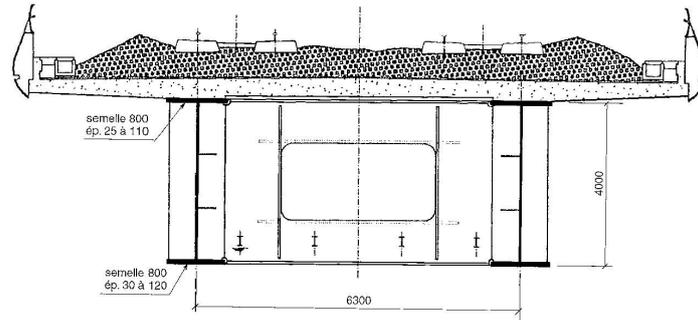
Simply supported



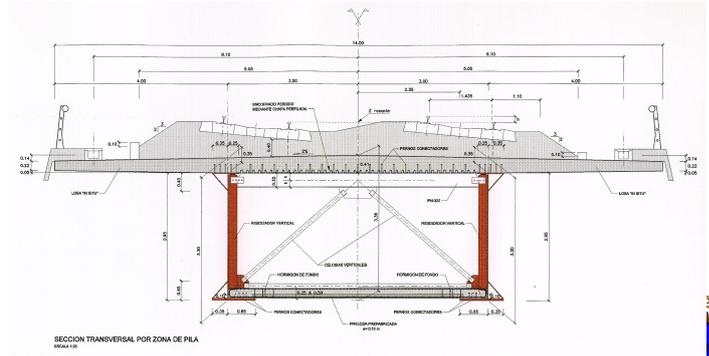
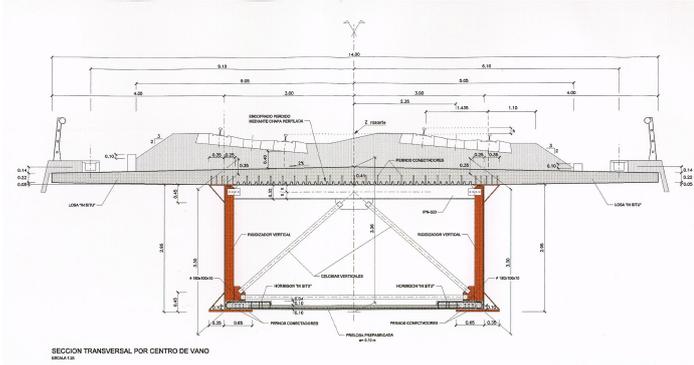
Not valid: $a_{max} > 3.5 \text{ m/s}^2$



Viaduct "Las Piedras" (F. Millanes, 2004)



Twin steel girder open section, **low torsional stiffness**



Partially closed section, **higher torsional stiffness**



Viaduct "Las Piedras" (II)

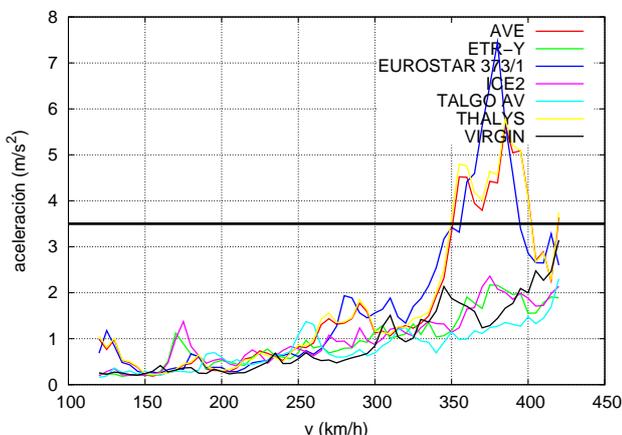
Total length 1208.5m, spans of $L = 63.5$ m, 92 m high pier



Viaduct "Las Piedras" (III). Acceleration envelopes

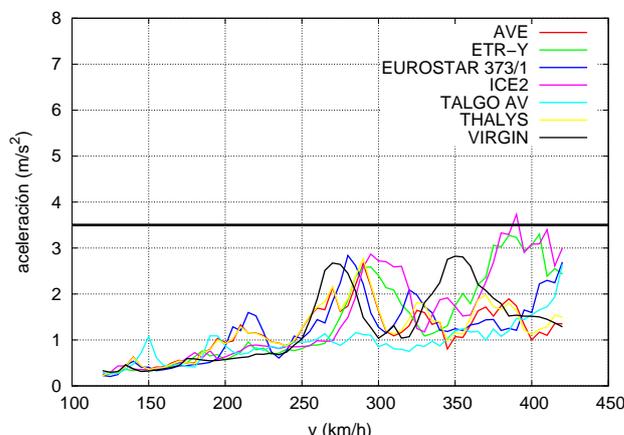
Vertical acceleration, w. bending and torsion, center of lateral span

Open section



Not valid: $a_{max} > 3.5 \text{ m/s}^2$

Partially closed section



Satisfies dynamic requirements



Design requirements for traffic safety

Vertical accelerations of deck

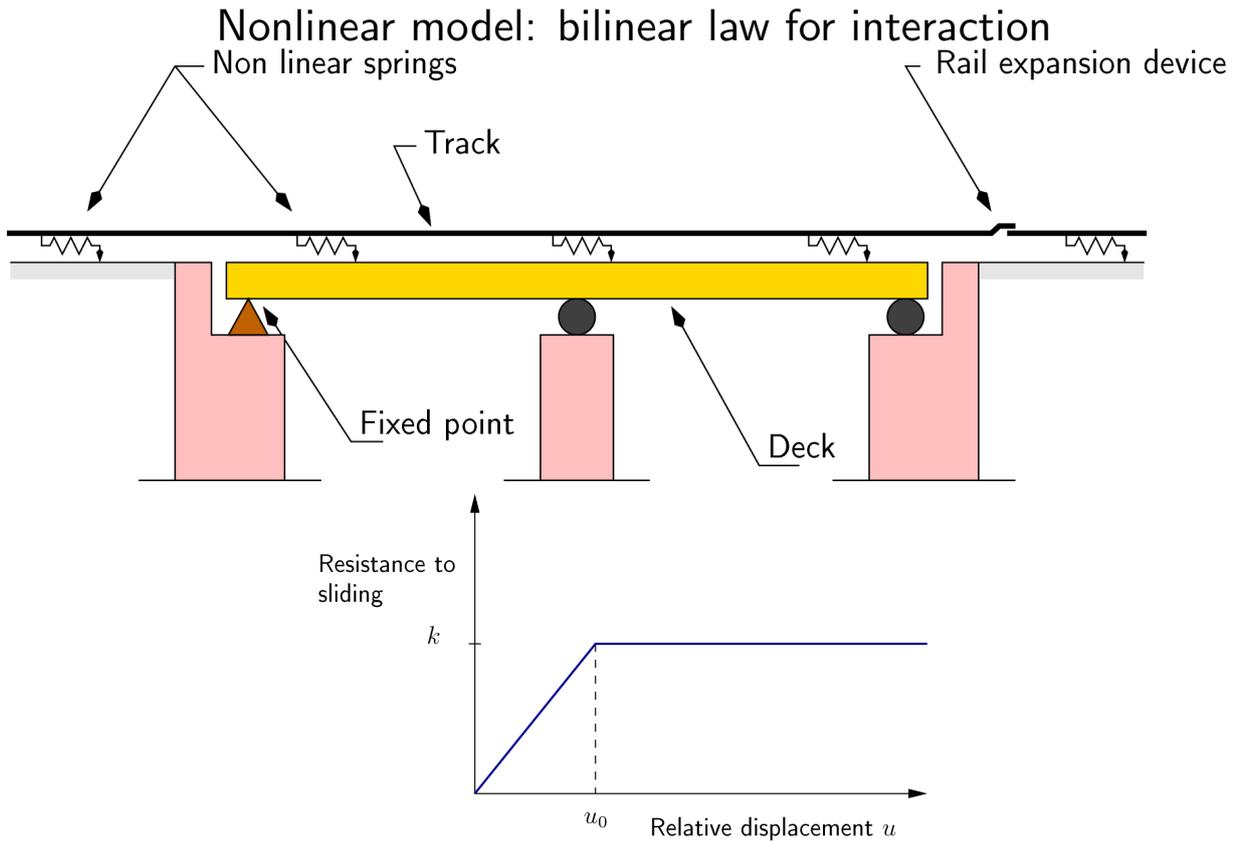
- For high levels of vibration ballast has been show to destabilize
- Requirement: $a \leq 3.5 \text{ m/s}^2$.

Track-Bridge Interaction

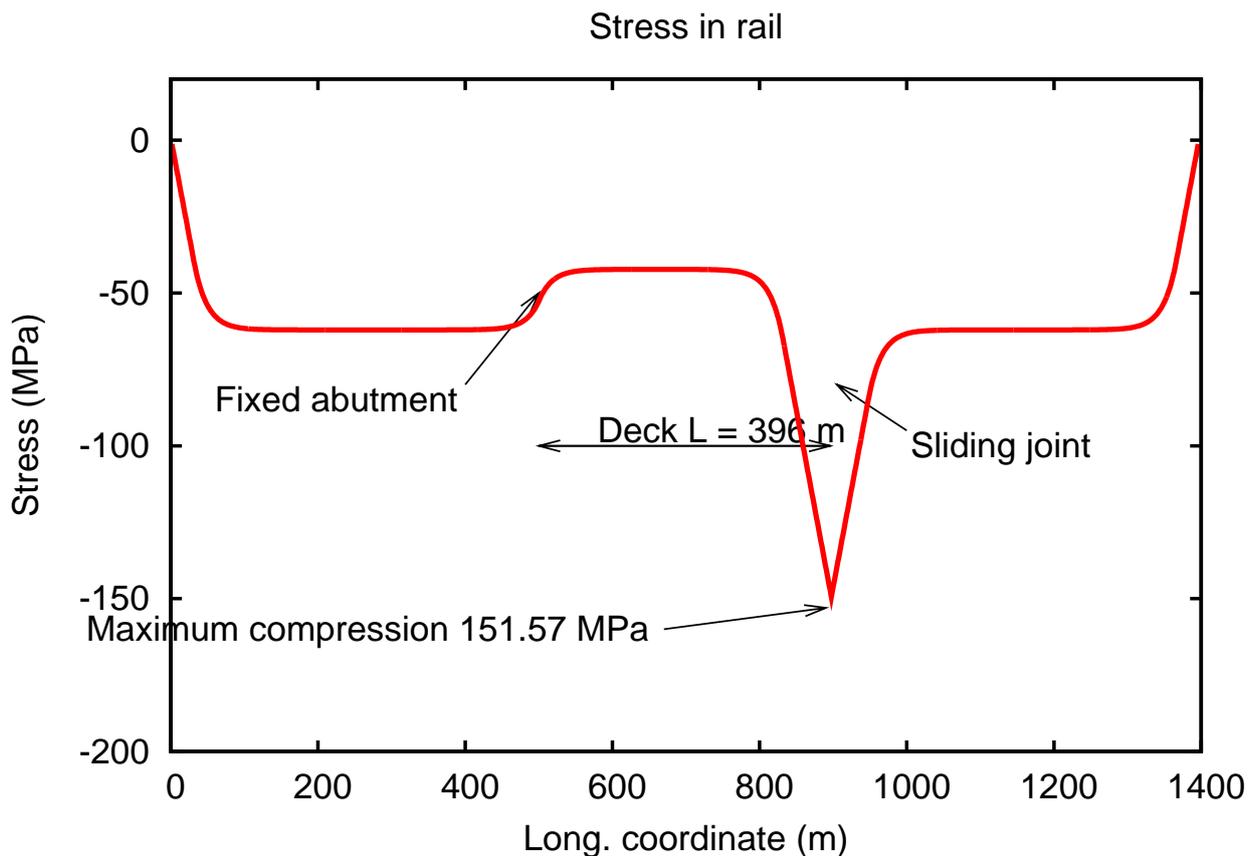
- **Long welded rail in bridges:** longitudinal loads from thermal actions, braking/acceleration carried by deck and rail;
- **Forces transmitted to piers and abutments** from combined actions of structure and track;
- **Rail stresses** due to thermal actions, braking and acceleration and other traffic loads;
- **Relative movements** and deformations at the ends of the deck due to the above variable actions.



Models to Consider for Track-Bridge Interaction

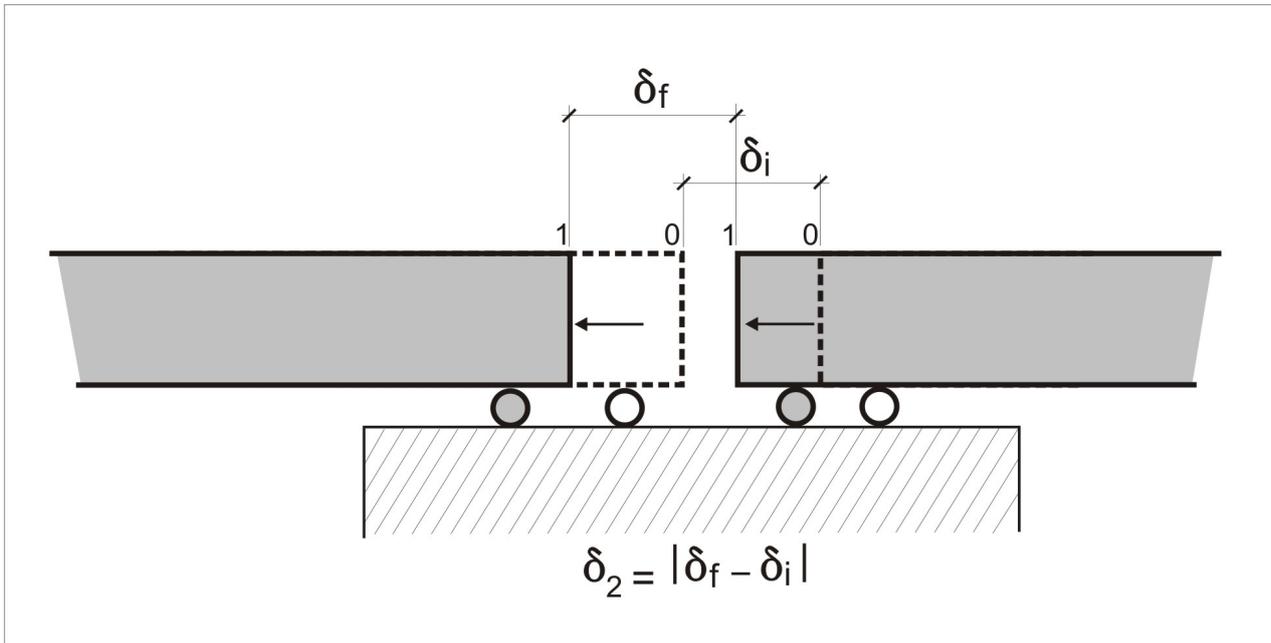


Additional Stress in Rail from Braking



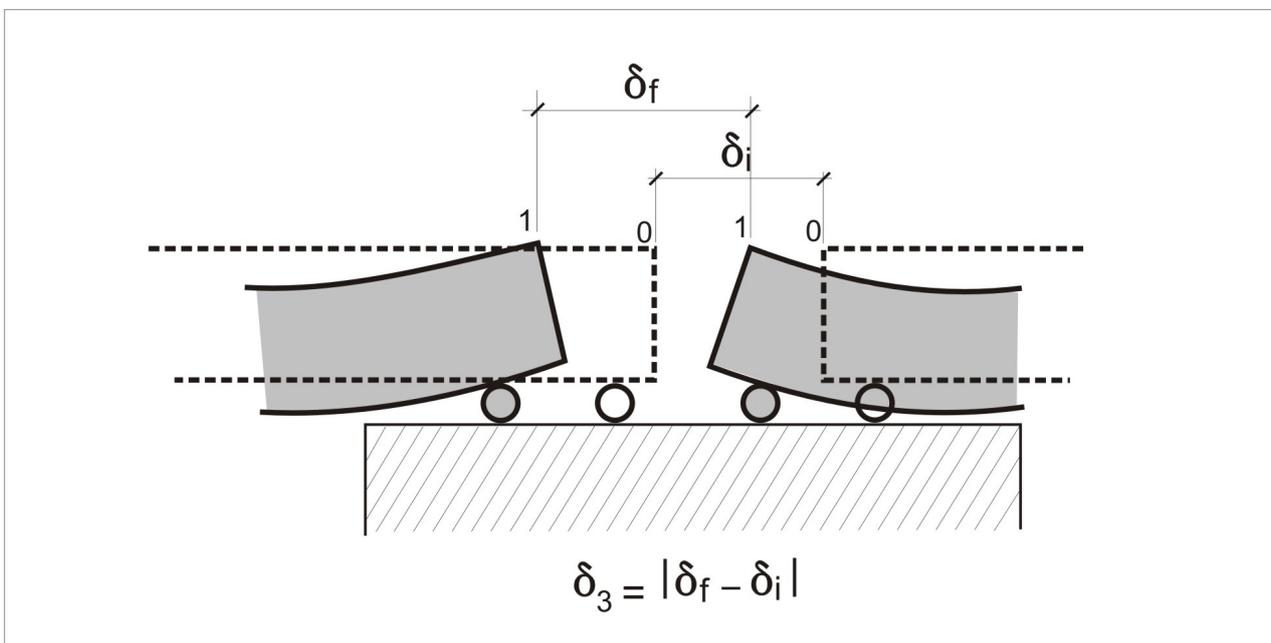
Horizontal Displacement from Braking/Acceleration

Relative displacement at deck joint: $\delta_2 \leq 5mm$



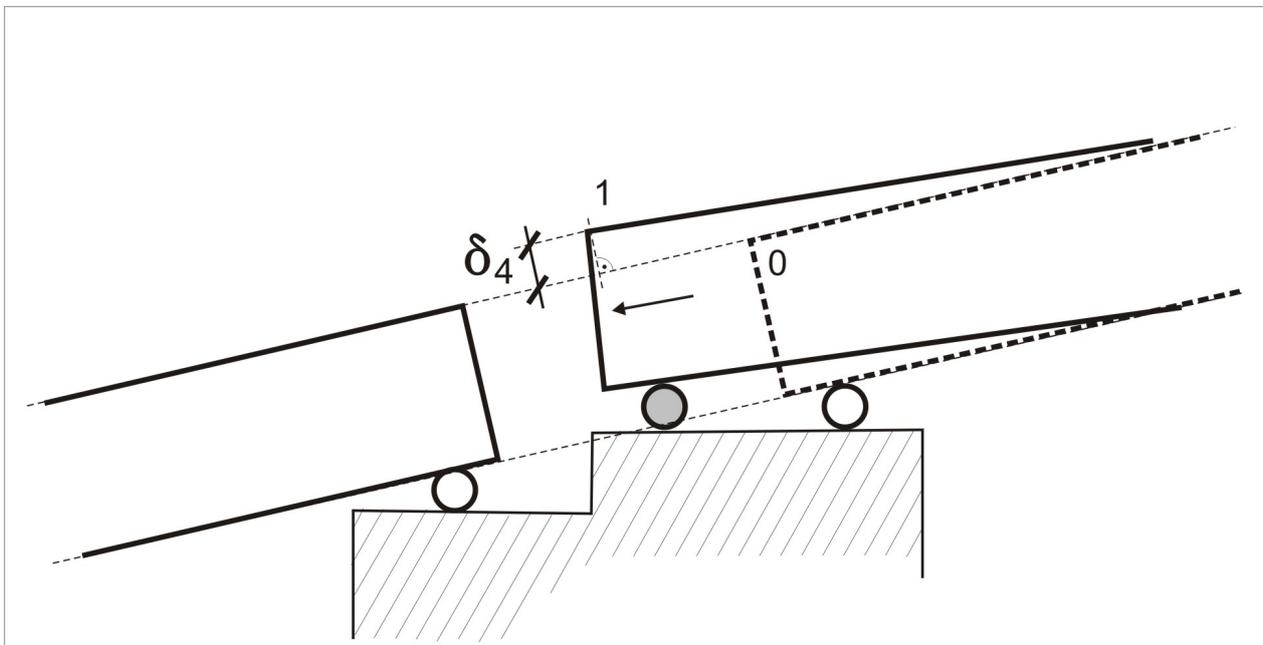
Horizontal Displacement from Bending due to Traffic Loads

Relative displacement at edge of deck joint: $\delta_3 \leq 5mm$

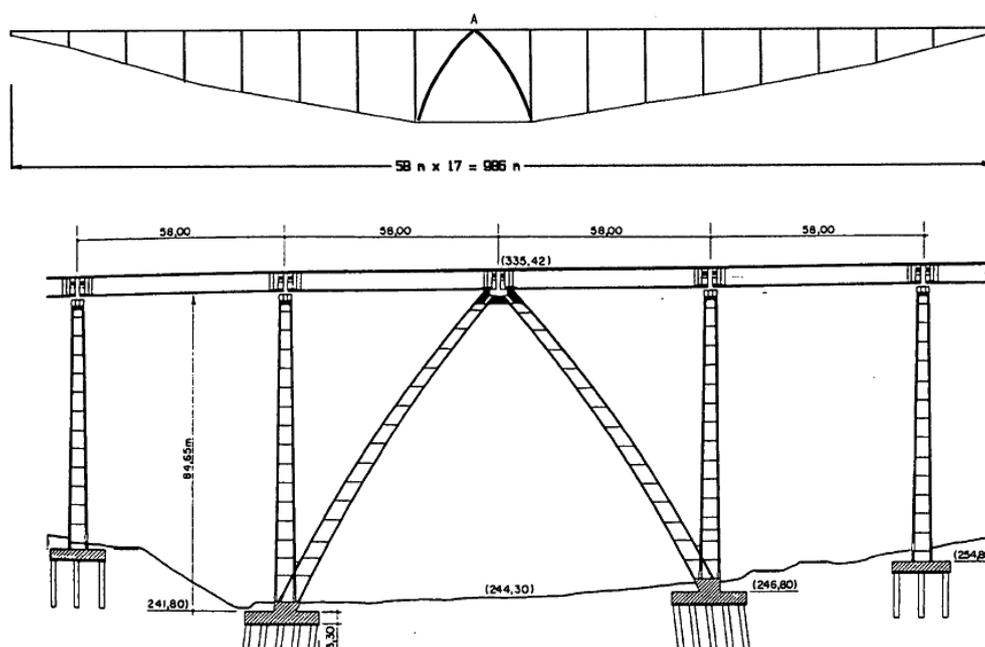


Vertical (Normal) Displacement at Edge from Variable Actions

Relative displacement at deck joint: $\delta_4 \leq 2mm$



Solution (example) for long viaduct: fixed point at center



Concluding Remarks

- **High Speed Railway (HSR) bridges** are a new and important type of infrastructure.
- **New phenomena** whose risk must be adequately considered: construction, dynamic effects, Track–Bridge interaction.
- **Dynamic analysis** is necessary for HS bridges to consider resonance
- **Serviceability Limit States (SLS)** for the structure are of utmost importance, as they become Ultimate Limit States for the safety of traffic
- **New codes** for actions in HSR bridges: EN 1991-2 2003, EN 1990-A1 2005, IAPF 2007



The End

THANKS FOR YOUR ATTENTION

Recognition

- **Coworkers/researchers:** J. Domínguez, J.A. Navarro, F. Gabaldón,
- **Master students:** F. Ruano, B. Sanz, A. Cámara, I. Barrios, R. Dias
- **Motivation:** J. Nasarre, E. Alarcón; Dir. General Ferrocarriles (J. Santos, I. Alonso, A. Corral)

